

Prediction of Torsional Strength for Very High Early Strength Geopolymer

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Very early high strength geopolymers are gaining acceptance as alternative repair materials for highways and other infrastructure. In this study, a very rapid geopolymer binder based on Metakaolin (MK) and Parawood ash (PWA), developed by the authors, was experimentally tested and a prediction model for its torsional strength is proposed. The geopolymer samples were subjected to uniaxial compression, flexural beam, and torsion tests. The modulus of rupture and torsional strength in terms of compression strength were found to be well approximated by $0.7(f'_c)^{1/2}$ and $1/7(x^2y)(f'_c)^{1/2}$, respectively. Also an interaction relation to describe combined bending and torsion was developed in this study. In addition, the effects of aspect ratio (y/x) were studied on both torsional strength and combined bending and torsion. It was found that an aspect ratio of $y/x = 3$ significantly reduced the torsional resistance, to about 50 % of the torsional strength of a square section.

Keywords: torsion, strength, geopolymer, modulus of rupture, concrete.

1. INTRODUCTION

In geopolymer synthesis, raw materials such as fly ash (FA), metakaolin (MK), and volcanic ash (VA) are mixed with concentrated alkaline solution under hydrothermal conditions. Normally these raw materials include alkali-activated alumino-silicate mineral binder. A 3-D network of Si-O-Al-O bonds is formed by polymerization in the alkaline environment [1–4]. Geopolymer binders can have excellent physical and chemical properties for a wide range of civil engineering and infrastructure applications, particularly in structural and non-structural repairs. Typically, concrete or cement based materials for structural repairs must provide high early compressive strength with rapid strength development and short curing time. Very rapid setting repair materials, such as very early high strength geopolymer binder, require a shorter period for setting and give high strength instantaneously at completion of the repair job. In highway and infrastructure repairs, there are economic costs associated with traffic closing or obstruction that need to be minimized, and potential new repair materials may provide effective solutions. Therefore, the geopolymers are gaining acceptance as alternative materials with economic advantages. Very high early strength geopolymer binders have widely studied and developed by many researchers [5–10]. Rapid setting time of the developed binders was particularly emphasized in these studies. To develop fast setting repair materials, modified cement with magnesia phosphate were experimentally evaluated [5, 6]. Properties and applications of cement mortar with magnesia-phosphate were studied by Yang *et al.* [7]. Comprehensive research on repair materials for concrete pavements is also available [6, 7, 9]. Modified Portland cement has been investigated at various mix proportions of PCC and latex

[8, 9]. Recently, high early strength geopolymers from metakaolin (MK) and Parawood ash (PWA) have been developed and evaluated for their mechanical properties by the current authors [10, 11]. In Southeast Asia, Parawood is mostly used as raw material for biomass power plants. Parawood ash (PWA) is one of residual products during the combination process of Parawood lumber and waste for electricity generation. Previous studies on the use of PWA as a material in geopolymer have still been rarely available [10, 11]. The feasibility study on Metakaolin based geopolymer containing PWA and Oil palm ash was investigated to develop high early compressive strength binder [11]. The geopolymer samples were tested for compressive strength and drying shrinkage, and for bond strength to Portland cement mortar with slant shear test. It was found that PWA significantly reduces drying shrinkage of test specimens [11]. Based on previous studies, knowledge concerning the mechanical properties and behaviors of geopolymer containing PWA are necessarily accomplished before field applications. Other properties such as modulus of rupture and torsional strength are still not well established even for normal strength geopolymer, and in particular not the high early strength variants. Experimental data on these properties are rare, so further research on modulus of rupture and torsional strength was necessary. Relations between the other evaluated mechanical properties and the compressive strength are assessed in the current study. In addition, the interaction of bending and torsion is subjected to a parametric study. This study will fill some gaps in the current knowledge about alternative repair materials, particularly for field applications.

2. MATERIALS

In this study, very high early strength geopolymer samples were prepared from the raw materials Metakaolin (MK), Parawood ash (PWA), river sand, sodium silicate,

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sodium hydroxide, and water. The Metakaolin was calcined at approximately 750 °C for 2 hours and ground in a ball mill to obtain MK particles with average size (d_{50}) of 6.31 μm . The PWA is a residual combination product from Para rubber wood, and was collected from biomass thermal power plants in southern Thailand. The PWA was ground in a ball mill for 4 hours. The particle size distributions of MK and PWA were evaluated by laser scattering and are shown in Fig. 1. To analyze the chemical composition of MK and PWA, X-ray fluorescence (XRF) spectrometry was used (Fig. 2). In general, the morphology of a geopolymer is mostly determined by the raw materials. The particle morphologies of MK and PWA were examined using scanning electron microscopy (SEM). It was found that both MK and PWA particles are mainly irregular in shape and of varying sizes.

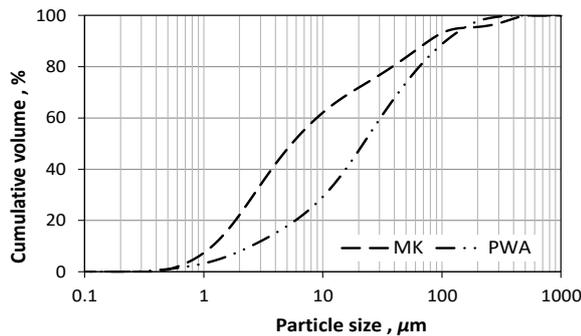


Fig. 1. Particle size distributions of the main mixture components

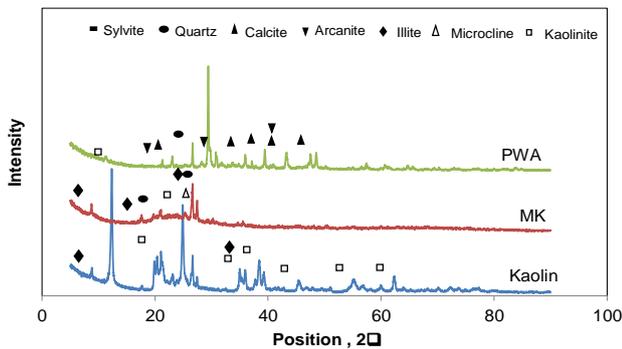


Fig. 2. XRD patterns of PWA, MK and natural kaolin

However, the PWA particles were the larger and rounder of the two, as shown in Fig. 3. In addition, the loss on ignition, specific surface area, and chemical composition are summarized in Table 1 for both MK and PWA. The main components in MK were 50 percent SiO_2 and 41 percent Al_2O_3 , confirming that it can be used in Si-Al cementitious materials.

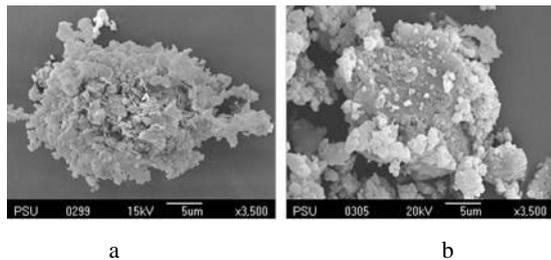


Fig. 3. SEM micrographs of the main raw materials: a – metakaolin (MK); b – parawood ash (PWA)

To ensure high alkaline activity in the geopolymer mix, the ratio of calcium oxide and magnesium oxide to silicon dioxide (SiO_2) should exceed one (EN 197-1, [12]). The main chemical components of PWA were 16 percent K_2O and 41 percent CaO (Table 1). It was found that the PWA used in this study had high alkalinity. The actual alkaline activator was prepared from sodium hydroxide (NaOH , 98 % pure flakes) with sodium silicate solution (14.1 % Na_2O , 27.7 % SiO_2 , and 56.3 % H_2O by weight) and water. The local river sand used as the fine aggregate had 4.75 mm maximum size and specific gravity 2.51. The 1.48 % water absorption of the fine aggregate was determined according to ASTM C 128 [13]. To obtain test samples with compressive strengths from 40 MPa to 70 MPa, four mixture recipes were designed and prepared varying the amounts of MK and PWA. The proportions of MK and PWA were designed based on the previous study by the authors [10].

Table 1. Chemical compositions and physical properties of the main raw materials

Oxide	PWA	MK
SiO_2	2.57	50.30
Al_2O_3	0.53	41.02
Fe_2O_3	0.56	1.05
CaO	41.19	0.33
TiO_2	-	1.50
MgO	4.52	-
K_2O	16.11	4.08
Other	10.78	-
Loss on ignition (LOI)	23.74	1.72
Specific surface area (m^2/g) BET	8.71	13.61
d_{10} , μm	2.860	1.352
d_{50} , μm	25.128	6.308
d_{90} , μm	123.450	88.803

Table 2. Mixture proportions and molar ratios in the mixture formulations

Mixture*	Proportion, %		Molar ratio			
	MK	PWA	$\text{SiO}_2/\text{Al}_2\text{O}_3$	CaO/SiO_2	$\text{Na}_2\text{O}/\text{SiO}_2$	$\text{H}_2\text{O}/\text{Na}_2\text{O}$
P0	100	0	2.77	0.01	0.21	11.43
P10	90	10	2.85	0.08	0.23	11.43
P20	80	20	2.95	0.16	0.25	11.43
P30	70	30	3.09	0.26	0.27	11.43

*(compressive strengths varied from 40 MPa to 70 MPa)

Note: All mixtures $\text{SS}/\text{N} = 2.5$, $\text{A}/\text{R} = 0.83$, $\text{W}/\text{R} = 0.45$, $\text{S}/\text{R} = 3$; where SS = sodium silicate, N = sodium hydroxide, A = sodium silicate + sodium hydroxide, W = water, S = sand, R = the raw materials as MK and PWA. High CaO content of raw materials causes large porosity [14]. Then, the increase porosity can lead to degradation of compressive strength. However, replacement of raw materials with PWA can reduce shrinkage rate of specimens (for specimens with heat curing at least 2 h) [11, 15].

Details of the ingredient proportions and molar ratios are shown in Table 2. Each batch was mixed to cast three test specimens each for compression, flexural, and torsion test, per formulation. The ingredients were weighted and mixed manually. The mixing procedure was as follows.

The alkaline activator (sodium silicate and sodium hydroxide) on mixing reached 74 ± 2 °C from reaction heat. Then the PWA and the fine aggregate were blended in with the alkaline activator, to form a fast setting product. For compressive testing, the fresh mixture was poured into an acrylic mold ($50 \times 50 \times 50$ mm) following the requirements of ASTM C109 [16]. The flexural and torsional tested samples were cast in a square prism (25×25 mm) with 285 mm length. The filled molds were sealed with polyvinyl sheets and cured at 80 °C temperature for 4 hours. After curing the polyvinyl sheets were removed, then the samples were remolded and left at ambient temperature (30 ± 2 °C) and relative humidity (70 ± 5 %) for 28 days before testing.

3. METHODOLOGY AND TESTING

The modulus of rupture, the torsional strength and the axial compression strength were determined by the flexural beam test under a concentrated load at the mid-span and the torsion test. For each sample batch, geopolymer mortar was cast into both cube and prism samples as mentioned in Section 2. The two main sets of experiments were: 1) compressive and flexural beam tests (samples for the same batch), and 2) compressive and torsional tests. The compressive strength of the cube samples was evaluated under uniaxial compression load, while the prism samples were subjected to flexural beam and torsion tests. The details of flexural beam and torsion tests are given as follows: To evaluate the modulus of rupture, prism samples ($25 \times 25 \times 285$ mm) were subjected to center point loading at the mid-span (ASTM C293 [17]) with a support span of 200 mm. The prism samples were subjected to a slowly applied load at the center of test span until failure. Schematic of the test set-up is shown in Fig. 4.

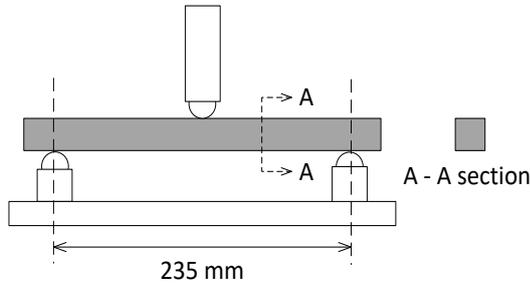


Fig. 4. Schematic set-up for testing prism samples

Modulus of rupture is defined as the failure stress under flexure. Typically, this failure stress is equal to or slightly lower than the failure stress in direct tension. In this study, the failure strength was obtained from the maximum applied sustained load before rupture. For a rectangular cross section, the modulus of rupture can be determined as:

$$f_r = \frac{3PL}{2bh^2}, \quad (1)$$

where, P is the maximum applied load, L is the test span, and b and h are width and thickness of the sample, respectively.

The prism samples were twisted until failure to evaluate the torsional strength of test samples. In this study, the test procedure concerned only Saint Venant's torsion induced by twisting a cross-section. The testing machine for torsion conducted the torque-twist tests as shown in Fig. 5.



Fig. 5. Torsion test of a prism sample

The prism samples were placed in the steel grips of rotating arms with 200 mm test span. The steel clips were clamped by applying torque to each bolt to prevent free body rotation within the steel grips. It should be noted that the cross sections at the prism ends were not constrained in the longitudinal direction, so free warping or horizontal moment was still allowed. Torque was applied into the test prism by anchoring one end and rotating the other end. The applied torque and torque reaction are of equal magnitudes but in opposite directions. An axial torsional load cell was attached to the anchored end to measure the reaction torque. The maximum applied torque was recorded and taken as the torsional strength of the test sample.

3.1. Torsional strength

When a bar is subjected to torsion, shear stresses are induced in the bar cross sections by the torsional moment. In a solid rectangular section the shear stress varies from the maximum at the centers of long edges to zero at the center of twisting. Under pure torsion, if the principal tension exceeds the tensile strength a diagonal crack will develop and eventually leads to failure. The torsional behavior of a bar before cracking can be predicted with the theory of elasticity. Based on St.Venant's elastic theory, the twist angle per unit length is directly proportional to the applied torque as in Eq. 2, and the maximum shear stress (τ_{max}) is given in Eq. 3.

$$\theta = \frac{T}{KG\alpha x^3 y}; \quad (2)$$

$$\tau_{max} = \frac{T}{\alpha x^2 y}, \quad (3)$$

where K is a correction factor accounting for the stiffness loss, G is shear modulus, α is St.Venant's coefficient depending on the ratio (y/x), and x and y are the short and long edge lengths of the rectangular section, respectively.

The torsional strength or cracking strength of a bar under pure torsion can be determined when the principal tensile stress reaches the tensile strength (f'_t).

$$T = \alpha x^2 y f'_t; \quad (4)$$

Based on plastic deformation theory, the torsional strength of a bar is still obtained when the principal tensile stress equals the tensile strength (f'_t) as well as the torsional strength from elastic theory as in Eq. 4. However,

St.Venant's coefficient is now replaced by the plastic coefficient (α_p). The torsional strength based on plastic deformation theory is given in Eq. 5.

$$T_p = \alpha_p x^2 y f_t'; \quad (5)$$

$$\alpha_p = \frac{1}{2} \left(1 - \frac{x}{3y} \right). \quad (6)$$

The skewed failure plane mechanism in plain concrete under torsion was introduced and verified by Lessig [18] and Hsu [19]. The modulus of rupture (f_r) was used as the critical stress. Later it was observed that the failure appeared in bending of a skew failure plane [20]. The torsional strength based on the skew-bending theory is presented in Eq. 7.

$$T = \frac{1}{3} x^2 y (0.85 f_r). \quad (7)$$

From Bredt's thin-walled tube theory, the torque on a cross section relates to shear flow (q) and its enclosed area (A_{cp}). For a plane rectangular section, the torsional strength or cracking moment can be obtained by replacing the original area (A_o) of the cross section with an equivalent thin-wall thickness ($0.75A_{cp}/P_{cp}$) and an enclosed area ($2A_{cp}/3$) as follows [21]:

$$q = \frac{\tau}{t} = \frac{T}{2 A_o}; \quad (8)$$

$$T = \left(\frac{A_{cp}^2}{P_{cp}} \right) \tau = \left(\frac{A_{cp}^2}{P_{cp}} \right) f_{cr}, \quad (9)$$

where T is applied torque, t is wall thickness, q is shear flow on a cross section, and f_{cr} is critical stress for torsional cracking.

4. RESULTS

4.1. Compressive strength and modulus of rupture

The results from the first set of experiments (compression strength and modulus of rupture) were assessed from a graph. For the 42 pairs of compressive and flexural data, the compressive strength and the modulus of rupture were evaluated, and are shown in Fig. 6. The ACI empirical formula for modulus of rupture was compared to the experimental results as shown in Fig. 6, and it was found that the modulus of rupture provided by ACI underestimated the experimental results in most cases. As found in several prior studies [1–4, 22, 23] the ACI empirical formula may be inappropriate for predicting the modulus of rupture. Thus, we looked for a more appropriate empirical formula based on the compressive strength. Regression analysis was used to find the best fits to the modulus of rupture (f_r). The empirical model proposed here for the modulus of rupture is as follows:

$$f_r = 0.70 \sqrt{f_c'}, \quad (10)$$

where f_c' is compressive strength (in MPa), and f_r is in the same units.

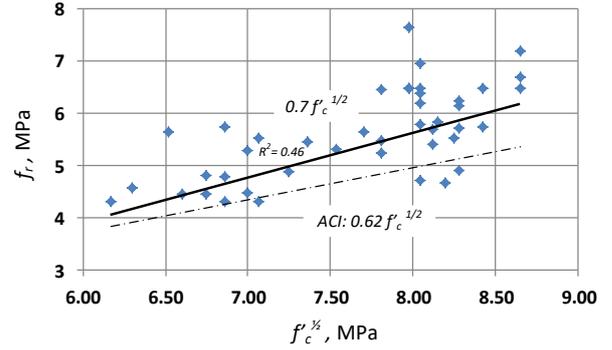


Fig. 6. Relationship of compressive strength – modulus of rupture

The difference in the modulus of rupture estimates from the proposed and from the ACI empirical formula is about 16%. The proposed model will here be used in the torsional strength model of Section 4.2.

4.2. Compressive and torsional strength

The prism samples ($25 \times 25 \times 285$ mm) under pure torsion exhibited a small inclined crack, initiated at the midpoints of the longer edges of a cross section. Sudden failure of the prism occurred when the maximum principal tensile stress exceeded the tensile strength of the material. The skewed and inclined failure plane is shown in Fig. 5. The correlation of the compressive and torsional strengths across 42 observations is presented in Fig. 7.

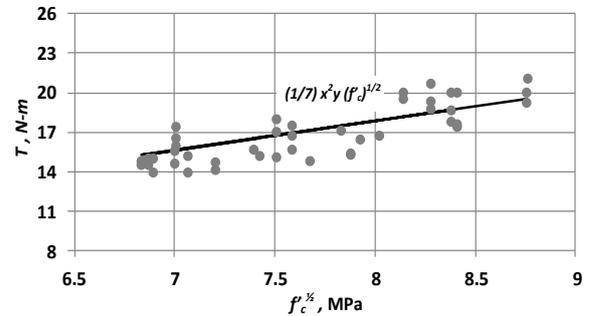


Fig. 7. Relationship of compressive and torsional strengths

A model based on the skew bending theory was developed in this study. For that model, the critical stress was taken as the tensile strength of the material. The coefficients for ordinary concrete in the relations between direct tensile strength, split tensile strength, and modulus of rupture were accepted in this current study, due to limited data on geopolymers. The approximate torsional strength based on the modified skew-bending theory for very early strength geopolymer bar is given in Eq. 11.

$$T = \frac{1}{7} x^2 y \sqrt{f_c'}, \quad (11)$$

where f_c' is the compressive strength (in MPa).

The approximate torsional strength is compared to the experimental data in Fig. 8.

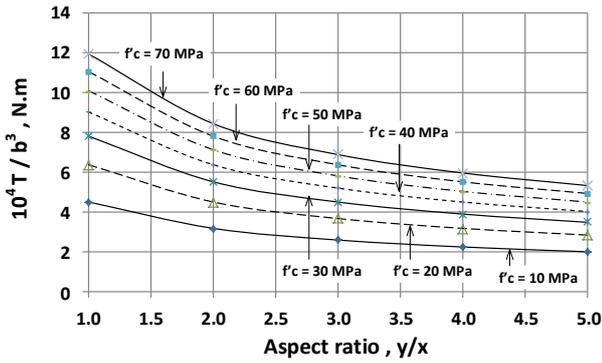


Fig. 8. Relation of torsional strength and aspect ratio (y/x). Note: the cross sectional area yx was held constant.

5. PARAMETRIC STUDY

To study effects of the aspect ratio (y/x) on torsional strength, this ratio was varied from 1 to 5 while holding the cross sectional area constant. From Eq. 11, the relation between torsional and compressive strengths suggests that the torsional strength should decrease as the aspect ratio (y/x) increases, while compressive strength is held fixed. The torsional strength indeed was significantly reduced by about 50 % as the aspect ratio (y/x) increased from 1 to 3, with slower decrease thereafter. The strength relations with aspect ratio are presented in Fig. 8 and Fig. 9.

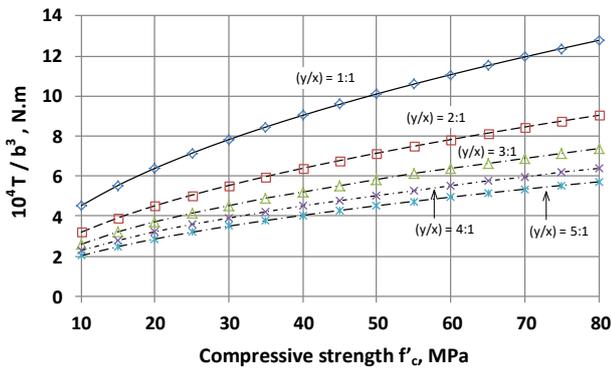


Fig. 9. Relation of torsional strength and compressive strength. Note: the cross sectional area yx was held constant

In general, the members are typically simultaneously subjected to different types of loading. The structural behavior under such combined loads is quite complex and developing an exact model of the structural response is not practical; but an approximate model of structural member responses to combined loads is necessary. The strength and structural responses could be represented by well-known approximations for interacting combined loads. From our experimental data, the interaction of bending and torsion was modeled using the proposed modulus of rupture and torsional strength models. The general interaction [18] is here modified as follows:

$$\left(\frac{8.57M^*}{y}\right) + \left(\frac{7T^*}{x}\right)^2 = 1; \quad (12)$$

$$M^* = \frac{M}{A\sqrt{f'_c}} \quad \text{and} \quad T^* = \frac{T}{A\sqrt{f'_c}}, \quad (13)$$

where f'_c is compressive strength (in MPa), A is cross sectional area (equaling xy), and M and T are applied bending moment and torque, respectively.

To assess the effects of aspect ratio on strength, the interaction relations in Eq. 12 and Eq. 13 are shown in Fig. 10. When the aspect ratio (y/x) of a cross section increases the torsional resistance significantly decreases under combined bending and torsion. An almost 50 % reduction in the torsional resistance was observed in comparison to pure torsion, for cases with aspect ratio exceeding 3.

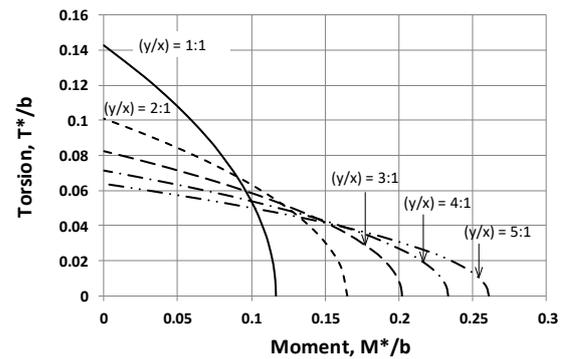


Fig. 10. Failure envelopes for interacting bending and torsion, as the cross-sectional aspect ratio is varied while its area is held fixed

6. CONCLUSION

Very high early strength geopolymer based on metakaolin (MK) and Parawood ash (PWA) that has been developed by the authors was experimentally evaluated for resistance to compression, flexural bending and torsion, to establish empirical models of torsion strength in terms of compressive strength. In this study, the modulus of rupture and the torsional strength were approximated by $0.7(f'_c)^{1/2}$ and $1/7(x^2y)(f'_c)^{1/2}$, respectively. The results from a parametric study showed that torsional strength can be reduced by 50 % when the cross section is shaped to aspect ratio 3 from original square shape, holding its area constant. In addition, interacting combined bending and torsion was studied and modeled. The torsional resistance is significantly decreased by the interaction of loadings, for cases with aspect ratio exceeding 3.

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